



TRANSMITTAL LETTER

Change 1 to Pub. 111
PennDOT Publication No. 111

January 6, 1977

BJECT:

Traffic Standards - Signing - TC - 7700 Series

INFORMATION AND SPECIAL INSTRUCTIONS:

Attached is an additional issue of PennDOT Construction Standards covering the erection of traffic signs, sign supports, overhead sign support structures, sign lighting and other traffic appurtenances. This particular issue is part of the TC-7718 group covering steel overhead sign support structures, spans 120 feet to 200 feet, with or without catwalks.

Direct any questions concerning these standards to Mr. J. R. Doughty, P.E., Director, Bureau of Traffic Engineering, Pennsylvania Department of Transportation, Room 1014, Transportation and Safety Building, Harrisburg, Pennsylvania 17120, Telephone number (717)-787-3620.

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APPROVED FOR ISSUANCE BY:

J. R. Doughty, P.E. Director

Bureau of Traffic Engr.

INFORMATIONAL NOTES

THESE NOTES MUST BE READ BEFORE USING THESE STANDARDS.

THESE STANDARDS SHALL BE USED FOR OVERHEAD STRUCTURES, SUBJECT TO LIMITATIONS AS SHOWN. THEY SHALL BE USED AS THE BASIS FOR THE PREPARATION OF STRUCTURE LAYOUTS AND CONTRACT PLANS. DETAILS SHOWN ON THESE STANDARDS NEED NOT BE COPIED, FOR REFERENCE TO THESE STANDARDS ON OVERHEAD SIGN STRUCTURE CONTRACT PLANS WILL BE PERMITTED. PROVIDING COORDINATING INFORMATION IS SHOWN ON THE CONTRACT PLANS.

DESIGN COMPUTATIONS ARE NOT REQUIRED FOR ANY PORTION OF A STRUCTURE FOR WHICH THE INFORMATION IS TAKEN DIRECTLY FROM THE DESIGN TABLES CONTAINED IN THESE STANDARDS, PROVIDING THE RESTRICTIONS RELATING TO THESE DESIGN TABLES ARE NOT EXCEEDED.

GENERAL DESIGN INSTRUCTIONS

DESIGN TABLES INCLUDED IN THESE STANDARDS WERE DEVELOPED USING A COMPUTER PROGRAM AND ARE BASED ON THE DESIGN CRITERIA SHOWN ON THIS SHEET

TOWERS SHALL BE SET AS FAR FROM EDGE OF ROADWAY PAVEMENT AS CROSS SECTION GEOMETRICS AND/OR RIGHT OF WAY WILL PERMIT, WITH THE MAXIMUM DISTANCE TO CENTERLINE OF TOWER EQUAL TO 30 FEET. GUARD RAIL PROTECTION SHALL BE PROVIDED FOR ALL TOWERS, REGARDLESS OF DISTANCE OF SETBACK.

TOP OF FOUNDATION PEDESTAL SMALL BE SET A MINIMUM ABOVE SURROUNDING TERRAIN TO MINIMIZE HAZARD EFFECT OF CONCRETE PEDESTAL ABOVE GRADE TOP OF FOUNDATION FOOTING TO BE SET A MINIMUM OF 2¹6" BELOW TOP OF PEDESTAL WITH A 1¹0" MINIMUM COVER MAINTAINED OVER FOOTING AT ALL POINTS. CARE SHALL BE TAKEN TO ATTEMPT TO KEEP TOP OF FOOTINGS BELOW BOTTOM OF GUARD RAIL POSTS THAT MIGHT BE IN AREA OF FOOTING OR FREE OF ANY OTHER OBSTRUCTING UNIT SUCH AS A STORM SEWER. IF IT BECOMES NECESSARY TO LOWER A FOOTING TO THE EXTENT THAT THE HEIGHT OF PEDESTAL IS GREATER THAN 7 FEET, THE SCOPE OF THE DESIGN TABLES HAS BEEN EXCEEDED AND IT WILL BE NECESSARY TO DESIGN A FOUNDATION PEDESTAL AND FOOTING FOR THIS SPECIAL CONDITION.

THE TRUSS, SIGNS, LIGHT FIXTURES (AND CATWALK, IF USED) SHALL BE SET TO AN ELEVATION THAT WILL PROVIDE 17-6" MINIMUM VERTICAL CLEAR ABOVE THE HIGHEST POINT OF THE ENTIRE WIDTH OF THE ROADWAY PAVEMENT AND SHOULDERS PASSING UNDER THE STRUCTURE. GENERALLY THE BOTTOM OF ALL SIGNS ON A STRUCTURE SHALL BE SET TO THE SAME ELEVATION. IN THE CASE OF A STRUCTURE SPANNING DUAL ROADWAYS, WHERE THE DIFFERENCE IN ELEVATION BETWEEN THE HIGHEST POINT ON EACH DUAL ROADWAY IS GREATER THAN 2-6". THE BOTTOM OF ALL SIGNS OVER EACH DUAL ROADWAY SHALL BE SET TO THE SAME ELEVATION. WITH THE ELEVATION DIFFERENCE OF BOTTOM OF SIGNS OVER EACH ROADWAY BEING EQUAL TO THE DIFFERENCE IN ELEVATION BETWEEN THE HIGHEST POINT ON EACH DUAL ROADWAY.

THE TRUSS SHALL BE SET TO AN ELEVATION THAT PLACES THE CENTER OF THE TRUSS AT MID-HEIGHT OF THE DEEPEST SIGN OR A MAXIMUM OF 6 FEET ABOVE THE BOTTOM OF THE SIGNS. IN THE INSTANCES WHERE THE ULTIMATE SIGN AREA CRITERIA IS TO BE USED, SET THE TRUSS TO AN ELEVATION THAT PLACES THE CENTER OF THE TRUSS AT 6 FEET ABOVE THE BOTTOM OF THE SIGNS. IN THE INSTANCES WHERE THE STRUCTURE IS SPANNING DUAL ROADWAYS WITH ELEVATION DIFFERENCES GREATER THAN 2.6", AS DESCRIBED PREVIOUSLY, SET THE TRUSS TO AN ELEVATION THAT PLACES THE CENTER OF THE TRUSS AT 8.6" ABOVE THE BOTTOM OF THE SIGNS OVER THE LOWER ROADWAY.

THE DESIGN SIGN AREA TO BE USED FOR SELECTING MEMBER SIZES FOR EACH STRUCTURE SHALL BE DETERMINED FROM ONE OF THE FOLLOWING TWO CONDITIONS. THE FIRST CONDITION IS THE ACTUAL SIGN AREA TO BE PLACED ON THE STRUCTURE AT THE TIME OF ITS CONSTRUCTION. THE SECOND CONDITION IS FOR AN ULTIMATE SIGN AREA EQUAL TO THE WIDTH OF ROADWAY PAVEMENT UNDER THE STRUCTURE TIMES IZ FEET. THE TRAFFIC ENGINEER SHALL SPECIFY WHEN THE ULTIMATE SIGN AREA IS TO BE USED FOR DESIGN OF STRUCTURE. THE DESIGN SIGN AREA SHALL BE THE SIGN AREA COMPUTED FROM EITHER OF THE PREVIOUSLY DEFINED CONDITIONS, ROUNDED TO THE NEXT HIGHER AREA SHOWN IN THE DESIGN TABLES.

TOWER MEMBER SIZES, FOUNDATION PEDESTALS AND FOOTINGS SHALL BE SELECTED FROM THE APPROPRIATE DESIGN HEIGHT IN THE DESIGN TABLES. THERE ARE THREE DESIGN HEIGHTS (THE DIMENSION FROM TOP OF FOUNDATION PEDESTAL TO THE CENTER OF THE TRUSS) VARYING IN 5 FOOT INCREMENTS FROM 23 FEET TO 33 FEET. USE AS THE DESIGN HEIGHT, THE HEIGHT IN THE DESIGN TABLE NEXT, LARGER THAN THE ACTUAL DIMENSION FROM THE TOP OF FOUNDATION PEDESTAL TO THE CENTER OF THE TRUSS.

OVERHEAD SIGN STRUCTURES MAY BE CONSTRUCTED WITH OR WITHOUT CATWALK. THE TRAFFIC ENGINEER SHALL SPECIFY WHEN A CATWALK IS TO BE INCLUDED AS PART OF THE STRUCTURE.

DESIGN CRITERIA

DESIGN SPECIFICATIONS - DESIGN REQUIREMENTS OF 1975 AASHTO STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS EXCEPT AS NOTED HEREIN.

WIND SPEED - 80 MILES PER HOUR.

COEFFICIENT FOR HEIGHT ABOVE GROUND = 1.00

| SHAPE COEFFICIENT - TRUSS CHORDS = 2.86 | TRUSS WEBS = 2.86 | SIGN = 1.19 | CATWALK = 2.86 | TOMER = 2.86

CONCRETE -

f'c = 3,000 P.S.I.

REINFORCEMENT BARS - fs = 20,000 P.S.).

STRUCTURAL STEEL - $F_y = 36,000 \text{ P.S.I.}$

FATIGUE LOADING USED EQUALS 0.71 × WIND LOADING DUE TO DESIGN WIND PRESSURE. FATIGUE STRENGTH EQUALS 100 PER CENT OF ALLOWABLE STRESS RANGE CONSIDERING 2,000,000 CYCLES OF LOADING.

MAXIMUM FOUNDATION BEARING PRESSURE - 1.5 TONS PER SQ.FT.

LOCATION OF RESULTANT CENTER OF PRESSURE UNDER FOOTING - WITHIN MIDDLE ONE-HALF OF FOOTING DIMENSION PERPENDICULAR TO CENTER OF TRUSS AND WITHIN MIDDLE ONE-THIRD OF FOOTING DIMENSION PERPENDICULAR TO CENTER OF TOWER.

DEAD LOAD AND WIND LOAD RESULTING FROM CATWALK HAVE BEEN INCLUDED IN THE DESIGNS SHOWN IN TABLES ON SHEET 2.

Commonwealth of Pennsylvania
DEPARTMENT OF TRANSPORTATION
BUREAU OF TRAFFIC ENGINEERING

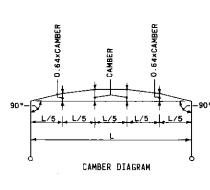
OVERHEAD SIGN STRUCTURE STANDARDS

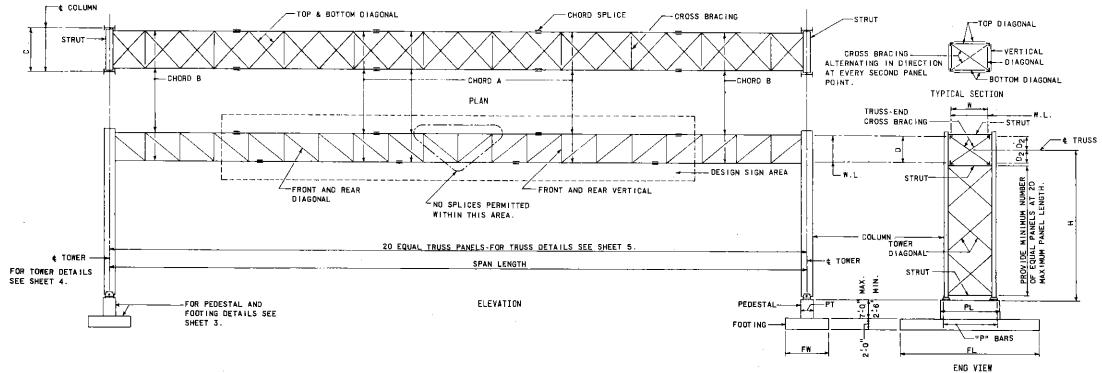
STEEL SPANS GREATER THAN 120 FEET
GENERAL INFORMATION

DESIGN INSTRUCTIONS AND CRITERIA

Recommended Recommended Private Approved to 125 / Sheet 1 01 1 Chief Bridge Engineer Director, Buredy of Traffic Engineering Highway Engineer TC-77

ESIGN	W×D	DESIGN				TRUSS MEA	MBERS							T(OWER MEMBERS			FOUNDATION PEDESTAL			FOOTING TYPE			DESIGN I	DES	
SPAN		AREA	1	ORDS	DIA	AGONALS	VERTICAL	CALS	S CROSS	BRACING CA	CAMBER		COLUMN	N .				NUMBER OF "P" BARS					AREA	SPA		
FT	FT.	SQ.FT.	A	В	FRONT &	TOP &	FRONT &		INTERIOR	EN	END		DES	IGN HEIGHT H	DIAGONALS	STRUTS	PL×PT	AND SIZE			H=23 FT.	. H=28 FT.	H=33 FT.	. sq.FT.	. Fi	
		34.11.	<u>.</u>		REAR	ВОТТОМ	REAR			<u> </u>		IN-	23 FT.	28 FT.	33 FT.			FT.	H=23 FT.	H=28 FT.	H=33 FT.		<u> </u>	ļ	34.71.	
130		300	L6×6×12	L6×6×12	L3 2 ×3 2 ×5	L3 2 ×3 2 ×5 16	L3 2 ×3	2 ×516	L3 2 ×3 2 ×5/6	L3 2 ×3	12 × 516		W24×68	₩24×68	W24×68	L4×4× ⁵ 8	C15x33.9	11x2.25	16#4	16#4	16#4	720	722	724	300	_
	7×5.25	700	L6×6×58	L6×6×12	 	ļ ļ		1				3.56	W24×68	W24×68	₩24×76	 _1		<u>t_</u>	16#4	16#4	16#5	821	824	727	700	_ '
		1100	L6×6×34	L6×6× 2									W24×76	W24×76	W24×84				16#4	16#5	16#5	922	826	730	1100	
		300	L6×6×916	L6×6×12			<u> </u>					4.19	W24×68	W24×68	W24×76		ļļ		16#4	16#4	16#4	720	722	724	300	
140	7×5.25	700	L6×6×34	L6×6×12									W24×76	W24×76	W24×84		ļ	<u> </u>	16#4	16#4	16#5	- 821	824	728	700	ı
_		1100	L6×6×78	L6×6× ⁹ 16									W24×84	W24x84	W27×94			11×2.25	16#5	16#5	16#5	923	826	731	1100	<u> </u>
	[300	ե6×6× ⁵ 8	L6×6×12								4.31	W24x68	W24×68	W24×76			12×2.25	16#4	6#4	16#4	720_	723	725	300	
150	8×6.0	700	L6×6×34	L6×6× 2									W24×76	W24×76	W24×84			1 1	16#4	16#4	16#5	822	824	728	700]
<u>.</u>		1100	L6×6×7g	L6×6×1 ₂			<u></u>						W24×84	W24×84	₩27×94 _€				16#5	6#5	16#5	923	826	731	1100	
160	8×6.0	300	L6×6×34	L6×6×12	1							4.69	W24×68	₩24×76	W24×76				16#4	16#4	6#4	819	723	725	300	
		700	L6×6× ⁷ 8	L6×6×12									W24×76	W27×84	W27×94				16#4	16#4	16#5	822	825	827	700	
	BXD.U	1100	L6×6×78	L6×6×9 ₁₆	L3 12 ×3 12 ×516							4.59	W27×84	W27×94	W27×102				16#5	16#5	16#5	923	926	928	1100	1
		1500	L6×6×1	L6×6×3 ₄	1312 ×312 ×38					T			₩27×102	W27×102	N27×114			12x2.25	16#5	16#6	16#6	1123	928	930	1500	7
Ī		300	∟6×6× ³ ₄	L6×6× 2	L3 2 ×3 2 ×5							4.81	W24×68	W24×76	W24×76			13×2.25	17#4	17#4	17#4	720	723	725	300	
170	9×6.75	700	L6×6× ⁷ 8	L6×6×12	L3 12 ×3 12 ×516								W24×76	W27×84	W27×94			1 1	17#4	17#4	17#5	921	825	827	700	
		1100	L6×6×1	L6×6× ⁹ 16	L3 12 ×3 12 ×38								W27×64	W27×94	W27×102			1	17#4	17#5	17#5	1022	926	928	1100	1
		1500	L6×6×1	L6×6×58	L312 x312 x38			Ì					₩27×102	W27×102	W27x114				17#5	17#5	17#5	1123	928	930	1500	1
	Ì	300	L6×6×7g	L6×6×12	L3 2 ×3 2 ×516							5.44	W24×76	W27x84	W27×94	1	 		17#4	17#4	17#4	820	822	824	300	+
`		700	L6×6×1	L6×6×9 ₁₆	L3 2 ×3 2 ×38		<u> </u>						W27×84	W27×94	W27×102			1	17#4	17#4	17#5	921	825	827	700	┨
180	9×6.75	1100	L8×8×34	L8×8×5 _B	L4×4×3 _B	1	L3 12 × 3	1 ₂ × 5 ₁₅					W27×94	₩27×102	₩27×114				17#5	17#5	17#5	1022	926	928	1100	7
		1500	L8×8×3	L.B×B×5	†				L3 12 ×3 12 ×516				W27×114	W27×114	W30×124			13×2.25	17#6	17#6	17#6	1123	1027	1030	1500	
		300	L6×6×7e	L6x6x12		1	L4×4		L4×4×3 _B				W24×76	W27×84	W27×94	1	 	14×2.25	8#4	18#4	18#4	820	823	825	300	+
190		700	L6×6×1	L6×6×9			1	,	1			5.63	₩27×94	₩27×94	W27×102			1	18#4	8#4	18#5	921	924	827	700	┪
	10×7.5	1100	L8×8×3	L8×8×5 ₈			1				_			W27×102	W27× 14	† †	† -		18#5	18#5	18#5	1022	926	928	1100	190
		1500	L8×8×3	L8×8×5 _B		 	_						N30×116		W30×124	1	 		18#5	18#5	18#5	1123	1028	1030	1500	
	-	300	L6×6×1	L6×6×9 ₁₆									W27×84	₩27×94	W27×102	 	1		8#4	18#4	18#4	919	823	825	300	+
	10-7-	700	L8×8×34	L8×8×5	1	 	1 -			t t	_		W27×94	W27×102	W27×114	 -	+	 	18#4	18#5	18#5	922	925	927	700	_
200	10×7.5	1100	L8×8×3	L8×8×5	1		<u> </u>	 				6.31	W27×114	+	W30x116	l	 	 	18#5	_	18#5	1122	1026	1028	1100	\dashv
		1500	L8×8×7 ₈	L8×8×5 _p	L4×4×30	L3 12 ×3 12 ×516	L4x4	. 7	L4×4× ³ 0	L3 2 ×3	1 6	_		W30x 124	W30×124	L4x4x ⁵ a	C15×33,9	14×2.25	18#5	18#5	1885	1122	1026	1030	+	1500





NOTES:

FOR GENERAL INFORMATION, DESIGN INSTRUCTIONS AND CRITERIA SEE SHEET 1.

FOR GENERAL NOTES SEE SHEET 3.

FOOTING TYPE DENOTES SIZE OF FOOTING. FOR EXAMPLE, TYPE 825 IS A FOOTING 8 $^{\prime}$ (FW) × 25 $^{\prime}$ (FL).

W.L. DENOTES WORKING LINE.

SEE TOWER ELEVATION ON SHEET 4 FOR METHOD OF COMPUTING DIMENSION C.

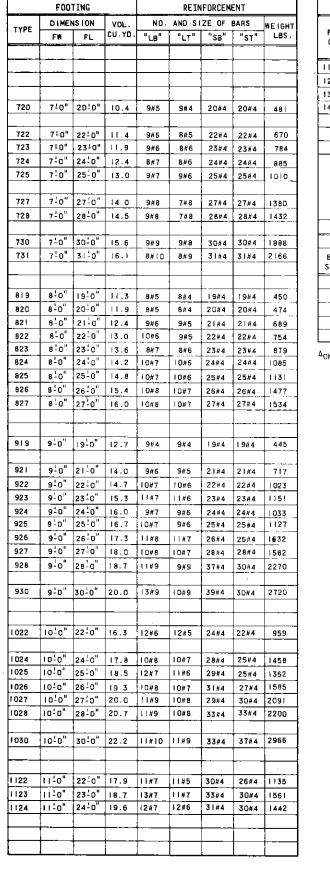
SPLICES MAY BE RELOCATED, ADDED OR ELIMINATED WITH APPROVAL OF THE TRAFFIC ENGINEER. RELATIVE LOCATIONS OF SPLICES IN TOP AND BOTTOM ORDER SHALL BE MAINTAINED AS INDICATED IN THESE STANDARDS. CHORD A SIZES SHALL BE EXTENDED TOWARD TOWERS, IF A SPLICE IS RELOCATED NEARER TO TOWER THAN SHOWN IN THESE STANDARDS.

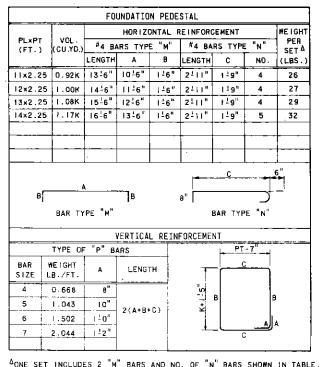
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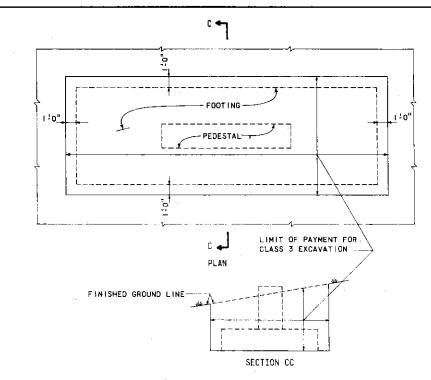
OVERHEAD SIGN STRUCTURE STANDARDS
STEEL SPANS GREATER THAN 120 FEET

DESIGN TABLES

Recommended	Recommended 19/21/24	Approved 10/13/16	Sheet 2 OF 7
BIKELEL	9RDowit	ויי בי חו	
	Director, Bureou of	Deputy Chief	TC-77
Ciliar Bridge Linginos.	Traffic Engineering	Highway Engineer	







ONE SET INCLUDES 2 "M" BARS AND NO. OF "N" BARS SHOWN IN TABLE.

OPTIONAL CONSTR.JT

L"LO"BARS

7.0"

FOOTING EXCAVATION DETAILS

"P" BARS

L"S8" BARS

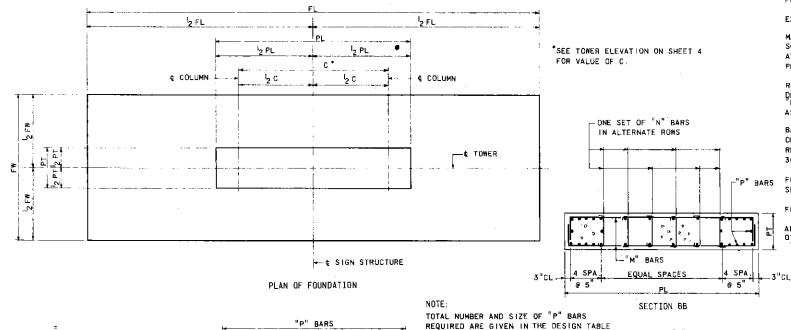
"L8" BARS

SECTION AA

—3"CL.-TYP.

F"ST" BARS

BARS



"ST" BARS

ELEVATION

"SB" BARS

ON SHEET 2.

CL.-TYP

__"LT"BARS

Commonwealth of Pennsylvania DEPARTMENT OF TRANSPORTATION BUREAU OF TRAFFIC ENGINEERING

OVERHEAD SIGN STRUCTURE STANDARDS STEEL SPANS GREATER THAN 120 FEET

FOUNDATION DESIGN AND DETAILS

Recommended 1921/16 Approved 10.128/16 Sheet 3 of 7 Director, Buredu of Traffic Engineering Highway Engineer hief Bridge Enginee

GENERAL NOTES:

STRUCTURAL SHAPES, BARS, PLATES, U-BOLTS, BENT BOLTS AND ANCHOR BOLTS

FOR GENERAL DESIGN INSTRUCTIONS SEE SHEET 1.

MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH

MATERIALS SHALL CONFORM TO THE FOLLOWING ASTM DESIGNATIONS:

NUTS FOR ANCHOR BOLTS

SPECIFICATION FORMS 408 AND 409.

H.S.BOLTS, NUTS AND WASHERS OTHER BOLTS

REINFORCEMENT BARS

A307

A36

A307

A325

A615, GRADE 40

NUTS FOR ALL BOLTS EXCEPT ANCHOR BOLTS SHALL BE HI-LOK® NUTS, AS MANUFACTURED BY THE HI-SHEAR CORPORATION, 2600 SKYPARK DRIVE, TORRANCE, CALIFORNIA, 90509; OR APPROVED EQUAL. THE HI-LOK NUT SHALL HAVE UNC-2B COARSE THREADS. PART NUMBERS FOR HI-LOK NUTS ARE AS FOLLOWS:

> HI-LOK NUTS FOR H.S. BOLTS - CHLII HI-LOK NUTS FOR OTHER BOLTS - CHL21

ALL HI-LOK NUTS SHALL BE INSTALLED WITH IMPACT WRENCHES OR WITH STANDARD HAND CLOSED-END WRENCHES OR SOCKETS. DURING INSTALLATION, THE HI-LOK NUT SHALL BE TORQUED UNTIL ITS WRENCHING HEX SHEARS OFF UNDER THE APPLIED TORQUE. AT THIS POINT, THE NUT IS SET AND NO FURTHER FORCE SHALL BE APPLIED TO THE REMAINING PORTION OF THE NUT OR TO THE HEAD OF THE BOLT.

ALL STEEL MATERIALS EXCEPT HI-LOK NUTS SHALL BE GALVANIZED AFTER FABRICATION IS COMPLETE. HI-LOK NUTS ARE ZINC-PLATED FOR USE WITH BOLTS GALVANIZED PER ASTM A-153.

CLASS A CEMENT CONCRETE SHALL BE USED IN FOOTINGS AND IN

EXPOSED CONCRETE EDGES SHALL BE CHAMFERED I" XI" EXCEPT AS NOTED.

MAXIMUM DESIGN FOUNDATION BEARING PRESSURE EQUALS 1.5 TONS PER SQUARE FOOT. THE FOOTING MAY BE ORDERED BY THE ENGINEER TO BE AT ANY ELEVATION OR OF ANY DIMENSIONS NECESSARY TO PROVIDE A PROPER FOUNDATION.

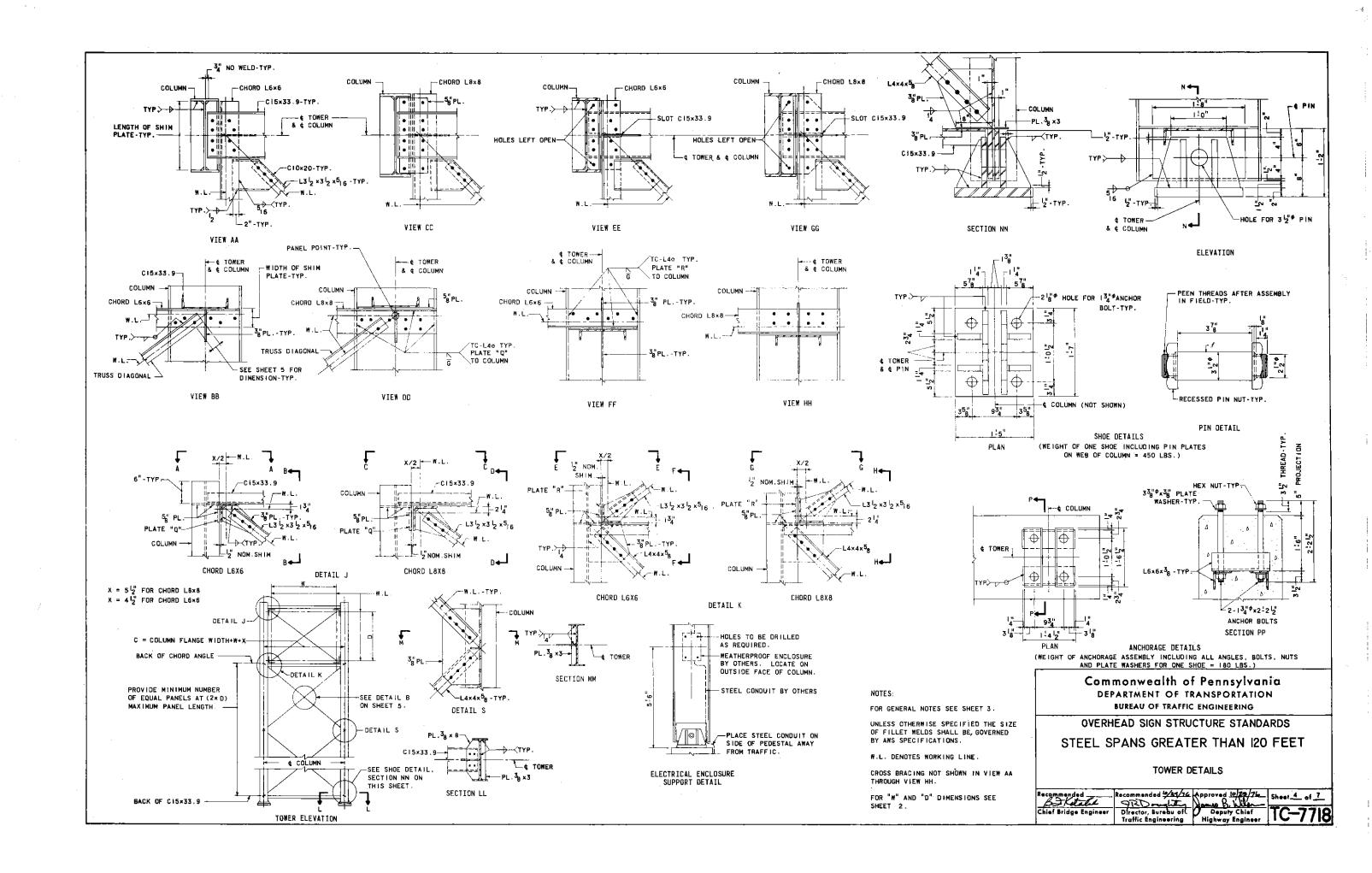
REINFORCEMENT BARS SHALL CONFORM TO THE DIMENSIONS SHOWN ON THE DRAWINGS AND WITHIN FABRICATING TOLERANCES AS SHOWN IN THE CURRENT MANUAL OF STANDARD PRACTICE FOR REINFORCED CONCRETE CONSTRUCTION AS PUBLISHED BY THE CONCRETE REINFORCING STEEL INSTITUTE.

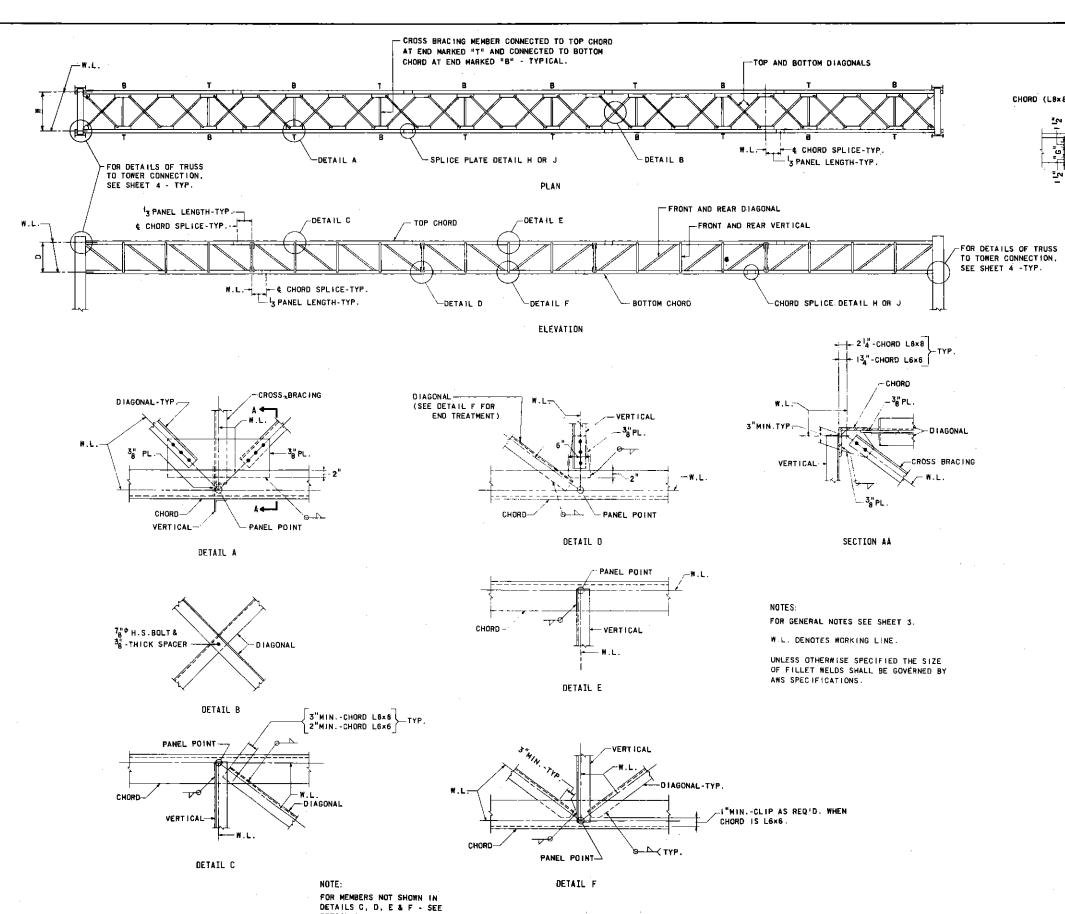
BARS SHALL NOT BE SPLICED EXCEPT AS PROVIDED ON THESE DRAWINGS OR AUTHORIZED BY THE ENGINEER. WHEN SPLICING IS APPROVED, THE REINFORCEMENT BARS SHALL BE LAPPED FOR A LENGTH OF AT LEAST 30 DIAMETERS AND SHALL BE SECURELY WIRED TOGETHER.

"P" BARS FOR PEDESTAL DIMENSIONS AND REINFORCEMENT, AND FOOTING TYPE, SEE DESIGN TABLES ON SHEET 2

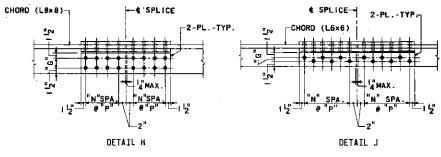
FOOTING SIZES SHOWN ARE MINIMUM REQUIRED FOR STABILITY.

ALL BOLTED CONNECTIONS SHALL BE MADE WITH $7_B^{\mu,\phi}$ H.S. BOLTS UNLESS OTHERWISE NOTED.





Capital Blue Print 17 0403-2007



CHORD SPLICE DETAILS

MINIMUM CHORD SIZE AT	SPLI	CE PL	ATES *	" _G "	"N"	"p"	WE IGHT
SPLICE POINT.	SIZE	NO.	DETAIL	(IN.)		(IN.)	Δ (LBS.)
Laxaxi ^l a	7 _{8 ×5} 3 ₄	4	Н	234	3	3 _	162
Lexexi	3 _{4 × 5} 3 ₄	4	н	234	3	3	142
L8×8×7 ₈	5 _{9 × 6}	4	Н	3	2	3	95
Lexex3 ₄	5 ₈ × 6	4	н	3	2	3	95
L8×8×58	^l 2 × 6	4	Н	3	2	3	79
L6x6x1	3 _{4 × 4}	4	Ţ	ı	5	212	123
L6x6x7 ₈	59 ×4 14	4	J	14	4	212	93
L6×6×3 ₄	2 ×4 12	4	J	1 12	4	2 4	81
L6×6× ⁵ e	12 ×4 12	4	ل	1 ¹ 2	3	214	62
L6×6×9 ₁₆	1 _{2 ×4} 1 ₂	4	J	1 2	3_	2 4	62
L6×6×12	38 × 4 12	4	J	1 2	2	2 4	38

- A INCLUDES WEIGHT OF SPLICE PLATES, H.S. BOLTS, HI-LOK NUTS AND WASHERS. DOES NOT INCLUDE WEIGHT OF FILL PLATES.
- * PROVIDE FILL PLATES AS REQUIRED TO MAINTAIN THICKNESS OF LARGER CHORD.

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OVERHEAD SIGN STRUCTURE STANDARDS
STEEL SPANS GREATER THAN 120 FEET

TRUSS DETAILS

	Recommended 225/26		76 Sheet 5 of 7
Chief Bridge Engineer	Director, Bureau of Traffic Engineering	Hames O. Wilson	TC-7719

